

March 15, 2024

Andrew Samson, EIT
 9220 Kimmer Drive, Suite 100
 Lone Tree, CO 80124

RE: Drainage Compliance Letter in support of Ambleside School project located at 9941 Lone Tree Pkwy (Existing Cornerstone Church).

Mr. Samson,

This drainage conformance letter has been prepared for Ambleside School located at 9941 Lone Tree Pkwy in Lone Tree, CO. The objective of this letter is to show the proposed site improvements will result in a de minimis increase in flows from the site and will not require additional storm sewer infrastructure improvements.

The existing Site is approximately 7.39 acres and is located on the southwest corner of Lincoln Avenue and Lone Tree Pkwy. The lot borders Lone Tree Pkwy to the east, Lincoln Ave to the north, single-family homes on the west (Highlands Ranch No. 90A), and single-family homes on the south (Block 3, Centennial Ridge Subdivision Filing No. 1). The existing site is currently occupied by the Cornerstone Church.

The proposed improvements will include installing 3,040 square feet of sidewalk throughout the Site and 616 square feet of asphalt paving. These proposed improvements will not alter the existing drainage patterns. However, since these proposed impervious areas will replace landscaped areas, there will be a small increase in overall imperviousness (approximately 1%) and runoff from the site.

The existing drainage calculations are provided in the Phase III Drainage Report from CKE Engineering, approved May, 4, 2012. For a comparison of the existing drainage conditions to the proposed conditions, please refer to the table below:

Sub-Basin	Existing Conditions					Proposed Conditions				
	Imp [%]	C ₅	C ₁₀₀	Q ₅ [cfs]	Q ₁₀₀ [cfs]	Imp [%]	C ₅	C ₁₀₀	Q ₅ [cfs]	Q ₁₀₀ [cfs]
A	53	0.41	0.61	8.6	23.1	54	0.42	0.61	8.8	23.2
B	74	0.57	0.70	4.2	9.6	75	0.58	0.71	4.3	9.7
C	64	0.48	0.65	3.4	8.2	64	0.48	0.65	3.4	8.2

Based on the results of the Rational Calculations, the proposed improvements will result in a small increase in flows during the 5-year and 100-year events. During the 5-year and 100-year events it is anticipated that an additional 0.3 CFS and 0.2 CFS would be generated, respectively. Improvements as part of Sub-Basin A are all that contribute to the existing on-site pond infrastructure and result in approximately a 1% and 0.50% change in flow rate for the 5-year and 100-year storms, respectively.

Conclusions:

While the proposed improvements will result in a small increase in flows during the minor and major storm events, this small increase in flows would have a negligible impact on the existing infrastructure and will not result in a need for additional storm sewer improvements. Additional future improvements to this site may result in the need for additional storm sewer improvements and drainage analysis. Further calculation would there for be required at that time to determine what, if any improvements, would be required.



Ambleside School – Drainage Letter
SWC Lone Tree Pkwy and Lincoln Ave
March 15, 2024

Sincerely,
GALLOWAY

Sid Biddle, PE
Civil Project Manager
SidBiddle@gallowayus.com

Appendices

- A. Existing Drainage Map
- B. Existing Rational Calculations
- C. Proposed Site Exhibit
- D. Proposed Rational Calculations

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March 13, 2024

APPENDIX A
Existing Drainage Map

CAD INFORMATION
 Paper Space Plot view(s)
 Model Space Plot view(s)
 Other View: PLAN
 Dwg. Name: D1.dwg
 Dwg. Number: 02/02/12
 Date: DECEMBER 12, 2011
 Job Number: J. COCO
 Design By: J. COCO
 Drawn By: J. COCO
 Checked By: J. COCO
 Sheet Number: D1

DESIGN POINT SUMMARY				
DESIGN POINT	CONTRIBUTING BASINS	Q5(CFS)	Q100(CFS)	Q100(CFS) FULL BUILD-OUT
1	POND OUTFALL	-	22.30	22.30
2	A, SINGLE FAMILY PARCEL, OFF-SITE BASIN OB	-	64.04	66.34
3	A AT DP3	1.51	3.98	N/A
4	B	4.23	9.55	9.55
5	C	3.36	8.22	8.22
6	A AT DP6	0.49	1.78	N/A
7	A AT DP7	2.97	12.15	20.23
8	A AT DP8	0.22	0.84	N/A </tr

BASIN SUMMARY						
BASIN		E(MIN)	C5	C100	Q5	Q100
A	CURRENT PHASE	7.3	.27	.56	5.59	20.83
A	FULL BUILD-OUT	7.0	.41	.61	8.62	23.13
B	DEVELOPED	5.5	.57	.70	4.23	9.55
C	DEVELOPED	5.5	.48	.65	3.36	8.22

CORNERSTONE CHURCH DRAINAGE MAP

LEGEND

	EXISTING		PROPOSED
	PROPERTY LINE		EASEMENT
	CURB & GUTTER		TYPE I (CATCH) CURB & GUTTER
	TYPE II (SPILL) CURB & GUTTER		HANDICAP RAMPS
	DRIVE		DRIVE
	INDEX CONTOURS		INTERMEDIATE CONTOURS
	DIRECTION OF FLOW		FUTURE PARKING

BASIN LEGEND

EXISTING BASIN BOUNDARY

BASIN BOUNDARY

EMERGENCY OVERTFLOW PATH

DESIGN POINT

BASIN

AREA IN ACRES

EXISTING BASIN

AREA IN ACRES

SWALE A
N.T.S.

100-YR WSE=0.43'

SWALE B
N.T.S.

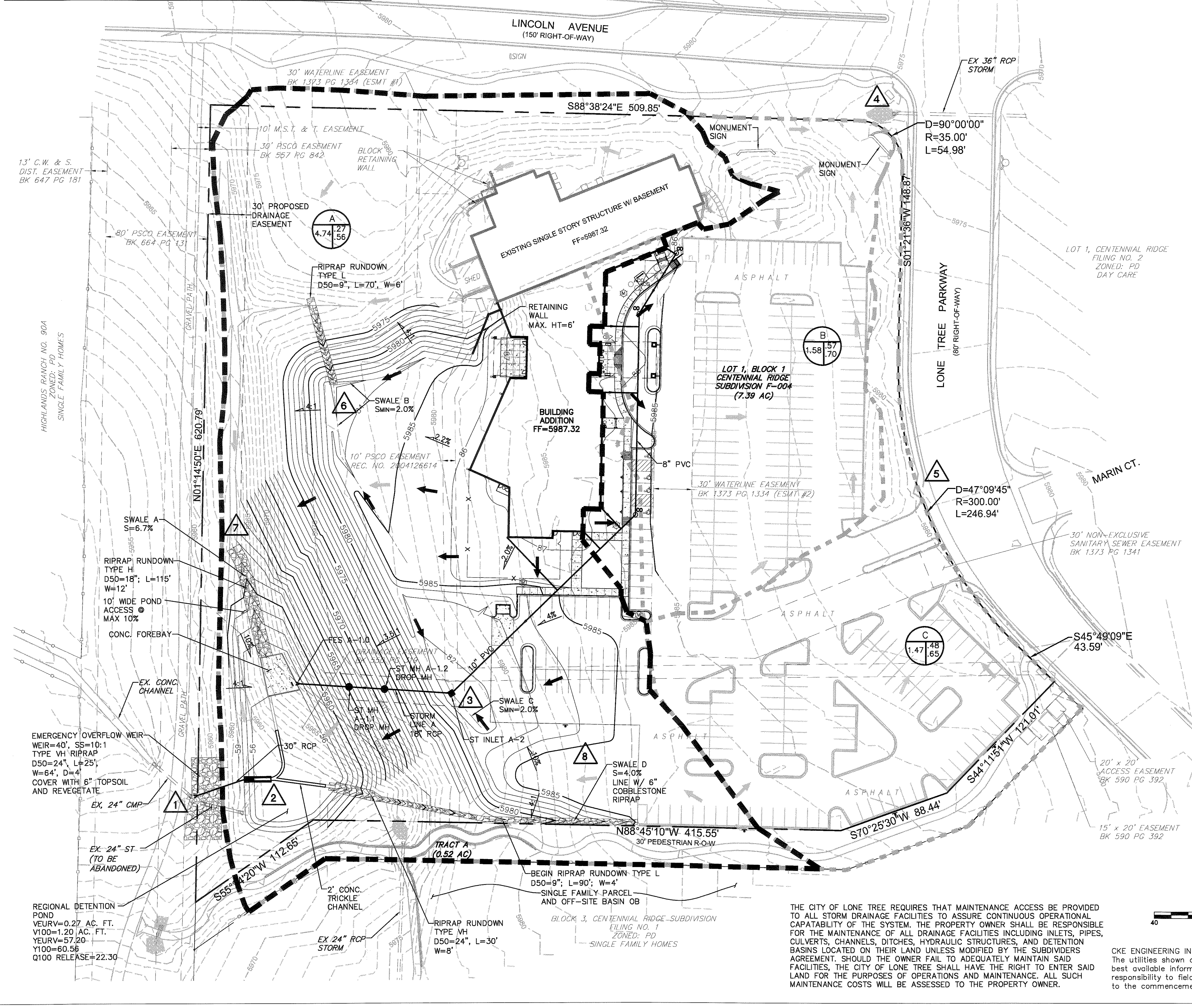
100-YR WSE=0.58'

SWALE C
N.T.S.

100-YR WSE=0.31'

SWALE D
N.T.S.

6" COBBLESTONE RIPRAP



THE CITY OF LONE TREE REQUIRES THAT MAINTENANCE ACCESS BE PROVIDED TO ALL STORM DRAINAGE FACILITIES TO ASSURE CONTINUOUS OPERATIONAL CAPABILITY OF THE SYSTEM. THE PROPERTY OWNER SHALL BE RESPONSIBLE FOR THE MAINTENANCE OF ALL DRAINAGE FACILITIES INCLUDING INLETS, PIPES, CULVERTS, CHANNELS, DITCHES, HYDRAULIC STRUCTURES, AND DETENTION BASINS LOCATED ON THEIR LAND UNLESS MODIFIED BY THE SUBDIVIDERS AGREEMENT. SHOULD THE OWNER FAIL TO ADEQUATELY MAINTAIN SAID FACILITIES, THE CITY OF LONE TREE SHALL HAVE THE RIGHT TO ENTER SAID LAND FOR THE PURPOSES OF OPERATIONS AND MAINTENANCE. ALL SUCH MAINTENANCE COSTS WILL BE ASSESSED TO THE PROPERTY OWNER.

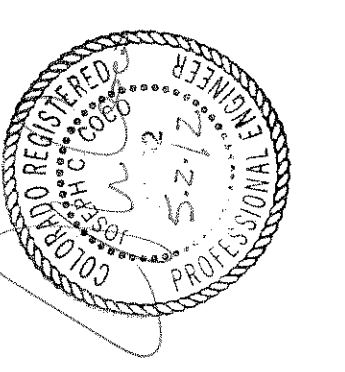
CKE ENGINEERING INC. assumes no responsibility for utility locations. The utilities shown on this drawing have been plotted from the best available information. It is, however, the contractors responsibility to field verify the location of all utilities prior to the commencement of any construction.

CORNERSTONE CHURCH DRAINAGE PLAN

No.	Description of Revisions	Date	Name
1	SUBMITTED TO CITY OF LONE TREE	02/02/12	JCC
2	REVISED PER CITY COMMENTS	04/06/12	JCC

Sheet Number: D1

CKE ENGINEERING INC
 14257 W. EVANS CIRCLE
 LAKEWOOD, CO. 80228
 (303) 917-1757



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Santa Fe & Phillips Ave
March 13, 2024

APPENDIX B
Existing Rational Calculations

WEIGHTED C-VALUE CALCULATION - FULL BUILDOUT

JOB NO: Full Buildout
 PROJECT: Cornerstone Church
 DATE: 3/30/2012

Soil Type (A-D): C

BASIN	TOTAL AREA (SF)	AREA LANDSCAPE (SF)	AREA PAVEMENT (SF)	AREA Roof Top (SF)	C2	C5	C10	C100	I (%)
ON-SITE BASINS									
A	206425	93146	71160	42119	0.36	0.41	0.47	0.61	53%
B	68626	17780	49531	1315	0.53	0.57	0.61	0.70	74%
C	63950	23257	40693	0	0.44	0.48	0.53	0.65	64%
DP7	170013	64875	71160	33978	0.41	0.46	0.51	0.63	60%
Total	509014	199058	232544	77412	0.40	0.45	0.51	0.63	59%

C Values are based on Urban Drainage Equations RO-6, RO-7 and Tables RO3 and RO-4.

Surface	i
landscape	0%
roof	90%
pavement	100%

STANSARD FORM SF-2
 TIME OF CONCENTRATION
 SUBDIVISION: Cornerstone Church
 CALCULATED BY: JCC

DATE: 3/30/2012

DESIG: (1)	SUB-BASIN DATA		INITIAL/OVERLAND TIME (t _i)				TRAVEL TIME (t _t)				t _c CHECK (URBANIZED BASINS)			FINAL t _c	REMARKS	
	C _s (2)	AREA Ac (3)	LENGTH Ft (4)	SLOPE % (5)	t _i Min (6)	LENGTH Ft (7)	SLOPE % (8)	VEL. FPS (9)	t _t Min (10)	COMP. t _c (11)	TOT. LENGTH Ft (12)	Min (13)	t _c Min (14)			
A	0.41	4.74	5	4.0	1.7	1005	2.8	3.2	5.2	7.0	1010	15.6	7.0			
B	0.57	1.58	35	10.0	2.6	490	2.1	2.9	2.8	5.5	525	12.9	5.5			
C	0.48	1.47	37	4.3	4.2	255	2.4	3.1	1.4	5.5	292	11.6	5.5			
DP7	0.46	3.90	5	4.0	1.6	980	2.8	3.2	5.1	6.7	985	15.5	6.7			

CALCULATED BY : JCC
 DATE: REV 3/30/2012
 CHECKED BY: JCC

STANDARD FORM SF-3
 STORM DRAINAGE SYSTEM DESIGN
 (RATIONAL METHOD PROCEDURE)

JOB NO: Full Buildout
 PROJECT: Cornerstone Church
 DESIGN STORM: 5 Year

BASIN	DESIGN POINT	DIRECT RUNOFF						TOTAL RUNOFF				STREET			PIPE				TRAVEL TIME			REMARKS
		AREA DESIGN (AC)	AREA (AC)	RUNOFF COEFF	t _a (MIN)	C * A (AC)	I (IN/HR)	Q (CFS)	t _c (MIN)	Σ (C * A)	I (IN/HR)	Q (CFS)	SLOPE (%)	STREET FLOW (CFS)	DESIGN FLOW (CFS)	SLOPE (%)	PIPE SIZE	LENGTH (FT)	VELOCITY (FPS)	t (MIN)		
	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)	(12)	(13)	(14)	(15)	(16)	(17)	(18)	(19)	(20)	(21)	(22)	
A	2		4.74	0.41	7.0	1.96	4.40	8.62					X	X	X	X	X	X	X	X	X	
B	4		1.58	0.57	5.5	0.89	4.75	4.23					X	X	X	X	X	X	X	X	X	
C	5		1.47	0.48	5.5	0.71	4.75	3.36					X	X	X	X	X	X	X	X	X	
DP7	7		3.90	0.46	6.7	1.78	4.45	7.92					X	X	X	X	X	X	X	X	X	

CALCULATED BY: JCC
 DATE: 3/30/2012
 CHECKED BY: JCC

STANDARD FORM SF-3
 STORM DRAINAGE SYSTEM DESIGN
 (RATIONAL METHOD PROCEDURE)

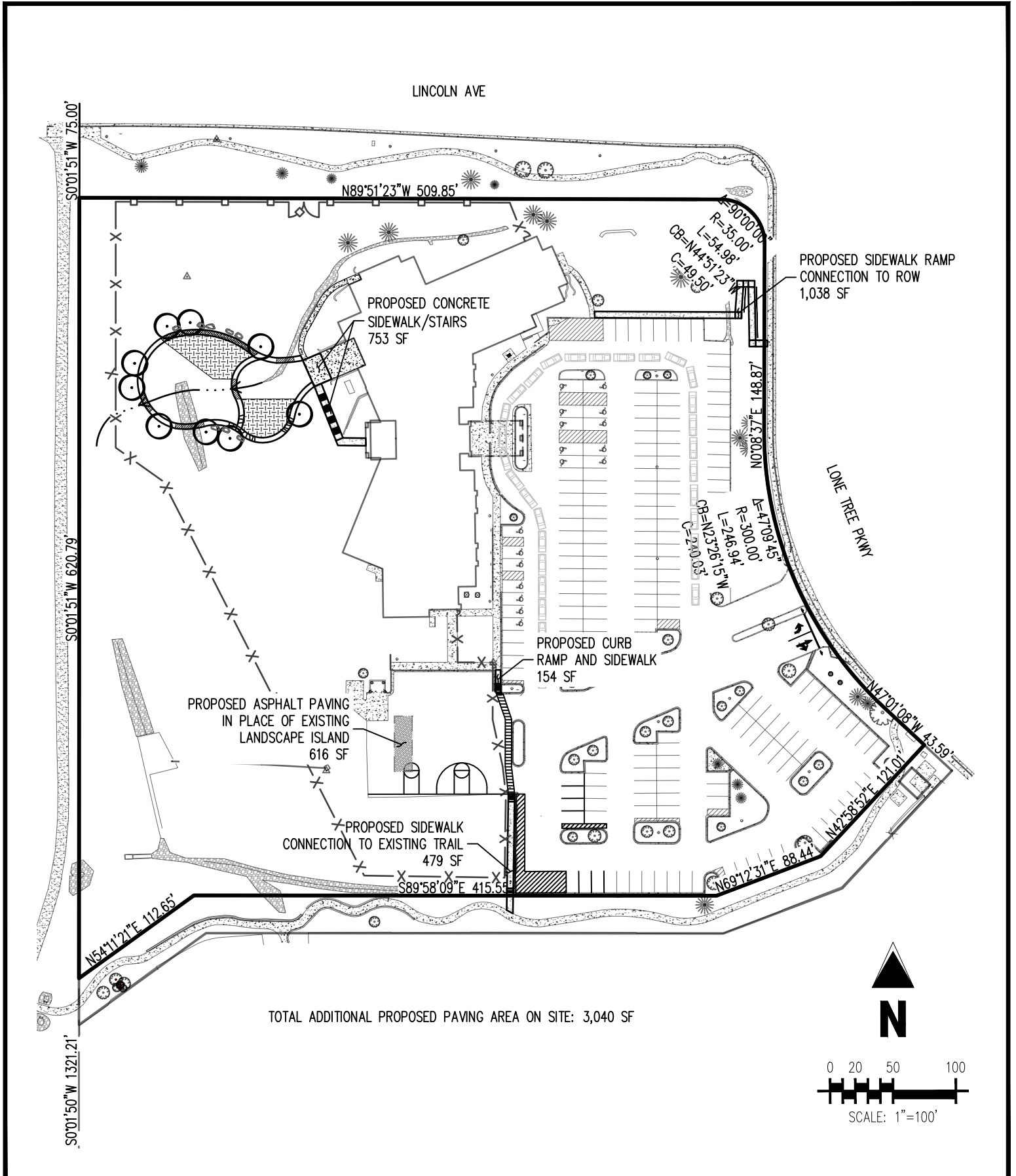
JOB NO: Full Bluidout
 PROJECT: Cornerstone Church
 DESIGN STORM: 100 Year

BASIN	DESIGN POINT	DIRECT RUNOFF						TOTAL RUNOFF				STREET			PIPE				TRAVEL TIME			REMARKS
		AREA DESIGN (AC)	AREA (AC)	RUNOFF COEFF	t _r (MIN)	C * A (AC)	I (IN/HR)	Q (CFS)	t _r (MIN)	Σ (C * A)	I (IN/HR)	Q (CFS)	SLOPE (%)	STREET FLOW (CFS)	DESIGN FLOW (CFS)	FLOW (CFS)	SLOPE (%)	PIPE SIZE	LENGTH (FT)	VELOCITY (FPS)	t (MIN)	
	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)	(12)	(13)	(14)	(15)	(16)	(17)	(18)	(19)	(20)	(21)	(22)	
A	2		4.74	0.61	7.0	2.89	8.00	23.13					X	X	X	X	X	X	X	X	X	X
B	4		1.58	0.70	5.5	1.10	8.65	9.55					X	X	X	X	X	X	X	X	X	X
C	5		1.47	0.65	5.5	0.95	8.65	8.22					X	X	X	X	X	X	X	X	X	X
DP7	7		3.90	0.63	6.7	2.47	8.20	20.23					X	X	X	X	X	X	X	X	X	X

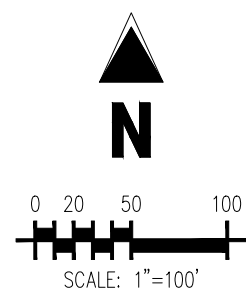
B-11

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APPENDIX C
Proposed Site Exhibit



TOTAL ADDITIONAL PROPOSED PAVING AREA ON SITE: 3,040 SF



AMBLESIDE SCHOOL

9941 LONE TREE PARKWAY
LONE TREE, CO 80124

SIDEWALK AREA EXHIBIT

Project No: AMC000002

Drawn By: DMH

Checked By: JSB

Date: 03/13/2024



5500 Greenwood Plaza Blvd., Suite 200
Greenwood Village, CO 80111
303.770.8884 • GallowayUS.com

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March 13, 2024

APPENDIX D
Proposed Rational Calculations

BASIN SUMMARY TABLE						
Tributary Sub-basin	Area (acres)	C ₅	C ₁₀₀	t _c (min)	Q ₅ (cfs)	Q ₁₀₀ (cfs)
A	4.74	0.42	0.61	6.99	8.8	23.2
B	1.58	0.58	0.71	5.50	4.3	9.7
C	1.47	0.48	0.65	5.50	3.3	8.2

COMPOSITE % IMPERVIOUS CALCULATIONS

Subdivision: Centennial Rdige
 Location: User Defined

Project Name: Ambleside School
 Project No.: AMC002
 Calculated By: ETA
 Checked By: JSB
 Date: 3/13/24

Basin ID	Total Area (ac)	Paved Roads/Sidewalks			Lawns			Roofs			Basins Total Weighted % Imp.
		% Imp.	Area (ac)	Weighted % Imp.	% Imp.	Area (ac)	Weighted % Imp.	% Imp.	Area (ac)	Weighted % Imp.	
A	4.74	100	1.68	35.4	0	2.09	0.0	90	0.97	18.40	53.8
B	1.58	100	1.16	73.7	0	0.38	0.0	90	0.03	1.70	75.4
C	1.47	100	0.93	63.6	0	0.53	0.0	90	0.00	0.00	63.6

STANDARD FORM SF-2
TIME OF CONCENTRATION

Subdivision: Centennial Rdige
Location: User Defined

Project Name: Ambleside School
Project No.: AMC002
Calculated By: ETA
Checked By: JSB
Date: 3/13/24

SUB-BASIN						INITIAL/OVERLAND (Sheet Flow)			Shallow Concentrated Flows					T _c CHECK			FINAL
DATA						(T _i)			(T _i)					(URBANIZED BASINS)			
BASIN ID	D.A. (AC)	Hydrologic Soils Group	Impervious (%)	C ₁₀₀	C ₅	L (FT)	S (%)	T _i (MIN)	L (FT)	S (%)	C _v	VEL. (FPS)	T _i (MIN)	COMP. T _c (MIN)	TOTAL LENGTH (FT)	Urbanized T _c (MIN)	T _c (MIN)
A	4.74	C	53.8	0.61	0.42	5	4.0	1.8	1005	2.8	20.0	3.2	5.2	7.0	1010.0	15.6	7.0
B	1.58	C	75.4	0.71	0.58	35	10.0	2.6	490	2.1	20.0	2.9	2.8	5.5	525.0	12.9	5.5
C	1.47	C	63.6	0.65	0.48	37	4.3	4.2	255	2.4	20.0	3.1	1.4	5.5	292.0	11.6	5.5

NOTES:

$$T_i = (0.395 * (1.1 - C_s) * (L^{0.5}) / ((S)^{0.33}), S \text{ in ft/ft}$$

$$T_i = L / 60V \text{ (Velocity From Fig. 501)}$$

$$\text{Velocity } V = C_v * S^{0.5}, S \text{ in ft/ft}$$

$$T_c \text{ Check} = 10 + L / 180$$

For Urbanized basins a minimum T_c of 5.0 minutes is required.

For non-urbanized basins a minimum T_c of 10.0 minutes is required

STANDARD FORM SF-3
 STORM DRAINAGE SYSTEM DESIGN
 (RATIONAL METHOD PROCEDURE)

Subdivision: Centennial Rdige
 Location: User Defined
 Design Storm: 5-Year

Project Name: Ambleside School
 Project No.: AMC002
 Calculated By: ETA
 Checked By: JSB
 Date: 3/13/24

STREET	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				STREET		PIPE			TRAVEL TIME			REMARKS
		Basin ID	Area (Ac)	Runoff Coeff.	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Slope (%)	Street Flow (cfs)	Design Flow (cfs)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	Tt (min)	
		A	4.74	0.42	7.0	1.99	4.41	8.8													
		B	1.58	0.58	5.5	0.91	4.74	4.3													
		C	1.47	0.48	5.5	0.70	4.74	3.3													



STANDARD FORM SF-3
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision: Centennial Rdge _____
 Location: User Defined _____
 Design Storm: 100-Year _____

Project Name: Ambleside School _____
 Project No.: AMC002 _____
 Calculated By: ETA _____
 Checked By: JSB _____
 Date: 3/13/24 _____

STREET	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				STREET		PIPE			TRAVEL TIME			REMARKS
		Basin ID	Area (Ac)	Runoff Coeff.	Tc (min)	C* A (Ac)	I (in/hr)	Q (cfs)	Tc (min)	C* A (Ac)	I (in/hr)	Q (cfs)	Slope (%)	Street Flow (cfs)	Design Flow (cfs)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	Tt (min)	
		A	4.74	0.61	7.0	2.89	8.03	23.2													
		B	1.58	0.71	5.5	1.12	8.63	9.7													
		C	1.47	0.65	5.5	0.95	8.63	8.2													

