PHASE I DRAINAGE REPORT AND PLAN FOR WILLOW CREEK PUD YOSEMITE ST. AND PARK MEADOWS DR.

LONE TREE, COLORADO

Prepared for:

Willow Creek PUD Park Meadows Dr. & Yosemite Dr. Lone Tree, Douglas County, CO

Contact: Vogel & Associates Phone: (303) 893-4288

Prepared by:



1526 Cole Blvd, Suite 100 Lakewood, Colorado 80401

Contact: Thomas Pannell, PE Phone: (303) 801-2900

> JN: 020460-01-001 April 2023

Signature Page

"This report (and plan) for the Phase I drainage design of Willow Creek PUD was prepared under my direct supervision in accordance with the provisions of the Douglas County Storm Drainage Design and Technical Criteria for the owners thereof. I understand that the City of Lone Tree does not and will not assume liability for drainage and erosion control facilities designed by others.

Thomas Pannell, PE State of Colorado No. 53615 For and on behalf of Bowman Consulting

Furniture Row Colo, LLC hereby certifies that the drainage facilities for Willow Creek PUD shall be constructed according to the design presented in this report. I understand that the City of Lone Tree does not and will not assume liability for the drainage facilities designed and/or certified by my engineer and that the City of Lone Tree reviews drainage plans pursuant to Lone Tree Municipal Code, Chapter 15, Article 1; but cannot, on behalf of Willow Creek PUD, guarantee that final drainage design review will absolve Furniture Row Colo, LLC and/or their successors and/or assigns of future liability for improper design. I further understand that approval of the Site Improvement Plan and Final Plat does not imply approval of my engineer's drainage design."

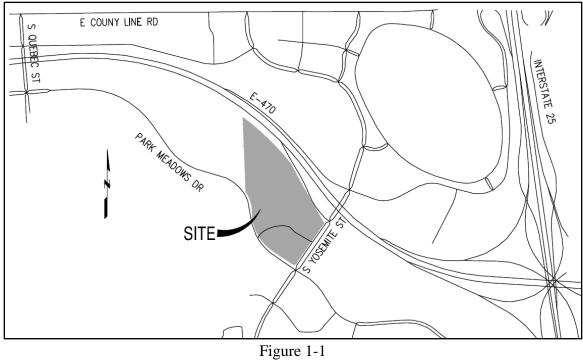
Name of Developer

Authorized Signature

1. General Location and Description

1.1 Site Location

Willow Creek PUD (hereafter, the Site) is located in the City of Lone Tree, Portions of the West half of Section 3 and the Northeast quarter of Section 4, Quarter Section NW ¹/₄, Township 6 South, Range 67W, Douglas County, Colorado. The Site is located in the northeast quadrant of the intersection of Park Meadows Drive and Yosemite Street. The Site consists of Tract E and BA of the Park Meadows Subdivision, Filing 1, 1st Amendment and is bound by C-470 to the north, Yosemite Street to the east, Park Meadows Drive to the south, and Willow Creek to the west. The Site is also bordered to the west by the undeveloped Tracts I(A) and I(B) of the Park Meadows Subdivision, Filing 1, 1st Amendment, which includes a 200' ingress/egress easement.



Vicinity Map

1.2 Description of Property

The total area of the property is 14.12 acres. The ground cover is mainly short grasses with some trees and shrubs. Existing grades on the Site are moderate, ranging from 0-20% and averaging around 4-7%. The slopes closer to Willow Creek are steeper, with some slopes over 30% near the creek banks. The Site generally slopes to the west towards Willow Creek.

The Site is within a portion of the Willow Creek watershed which is a tributary of Little Dry Creek. The majority of the watershed is developed according a Flood Hazard Area Delineation (FHAD) report for Willow Creek by CH2MHill completed for Urban Drainage and Flood Control District in 2010.

Soils for the site are classified as NRCS hydrologic soils groups D per the Web Soil Survey from the USDA Natural Resources Conservations Service.

A small portion of the Site is within a regulatory floodplain per the Douglas County and City of Lone Tree Flood Insurance Rate Map (FIRM) Panel 42 Map No. 08035C0042G.

There are no existing irrigation canals on the site. There are no stock ponds located on the site.

The current zoning is Commercial C3 for Tract E and Business for Tract BA. The proposed road access bridge to the site from Park Meadows is located within the Tract I(B) which is zoned for Parks and Open Space.

2. Drainage Basins and Sub-Basins

Existing Drainage Basins

Existing available drainage studies that impact the site are:

- Flood Hazard Area Delineation Willow Creek, CH2MHILL, December 2010.
- Flood Insurance Rate Map, Douglas County and City of Lone Tree, Panel 42 Map No. 08035C0042G.
- Drainage Analysis for Park Meadows/C-470 and Starika Property, Carroll and Lange, Inc., March 1996, Last Revised April 1999.
- Construction Plans for C-470/Yosemite Interchange, Felsburg Holt & Ullevig, December 1995.

See Appendix E for the above drainage studies.

Proposed Major Drainage Basins

Basin A1 (8.87 ac) covers most of the Site and encompasses the proposed residential units, driveways, and landscaping areas. It runs along the east side of Willow Creek. This basin consists of 52.1% landscaping area, 30.4% hardscape, and 17.5% roof area. The 5-yr runoff coefficient is 0.42, the 100-yr runoff coefficient is 0.68, and the overall imperviousness is 47.2%. Runoff will be collected in a detention facility. (Q5=7.71 cfs, Q100=25.77 cfs).

Basin A2 (4.49 ac) covers the south portion of the site along the north corner of Park Meadows Drive and South Yosemite Street. This basin consists of 33.9% landscaping area, 47.2% hardscape, and 18.9% roof area. The 5-yr runoff coefficient is 0.56, the 100-yr runoff coefficient is 0.75, and the overall imperviousness is 64.9%. Runoff will be collected in a detention facility. (Q5=7.60 cfs, Q100=20.92 cfs).

Basin UD1 (0.76 ac) covers undetained sections of the Willow Creek streambank that flow offsite into the stream. This area consists of 100% landscaping area and has a 5-yr runoff coefficient of 0.05 and a 100-yr runoff coefficient of 0.49. The overall imperviousness is 2.0%. (Q5=0.12 cfs, Q100=2.31 cfs).

3. Drainage Design Criteria

3.1 Development Criteria Reference

The Site was designed to comply with the Douglas County Storm Drainage Design and Technical Criteria Manual.

3.2 Hydrologic Criteria

The Site is located within the Willow Creek Tributary watershed which was studied in the FHAD Report previously mentioned completed by CH2MHill. The basins that encompass The Yard at Lone Tree are Basin 26 and Basin 23 of the FHAD study. Sub-watershed characteristics information from the FHAD can be found in Appendix E of this report. A drainage analysis was previously prepared for Park Meadows/C-470 and Starika Property by Carroll & Lange, Inc in 1999 which examined the properties within the watershed upstream of the Site. Computations from this study were utilized to determine the flows of the 66" pipe which currently runs through the site. The existing pipe will be rerouted to avoid proposed site features, maintain adequate capacity to convey the existing flows, and will outfall to Willow Creek in generally the same location as they exist today. Large retaining walls along the western property line of the Site, which is also the low side of the site, constrain the ability to meet WQCV, EURV, and 100-year detention facility. A 100-year detention facility will be located on-site to provide water quality, EURV, and 100-year detention. Appendix F outlines the potential areas where above ground and below ground detention facilities could be implemented.

3.3 Hydraulic Criteria

The Rational method was used to determine historic and developed flow rates. Runoff was calculated for the 5 and 100-year events. Times of concentration, rainfall intensities, and runoff coefficients were determined using the Douglas County Storm Drainage Design and Technical

Criteria Manuals and Urban Drainage and Flood Control District Manual, Volume 1. See Appendix A for Hydrological Calculations.

4. Drainage Facility Design

4.1 General Concept – Proposed Stormwater Conveyance/Storage Facilities

Proposed on-site stormwater conveyance will consist of a series of storm inlet connections to a full spectrum detention facility, that outfall into Willow Creek. Flows from the entire site will drain to this detention facility with a detention volume of 1.162 acre-feet. This detention volume may be in one or multiple ponds that will be determined at a later phase of development. See Appendix D for sizing calculations for the proposed pond.

Emergency overflow paths will be incorporated into the development. In particular, paths will be provided at the low points in the development to allow water to leave the Site during events exceeding the 100-year event, or if a failure occurs in the stormwater systems. Additionally, 1 foot of freeboard will be provided for the full spectrum detention pond with an overflow weir to allow for storm events larger than the 100-year event to safely pass through the facilities.

Proposed Storage Facilities								
ID	Area	Imperviousness	EURV	WQCV	100-YR			
	(acres)		(acre-ft)	(acre-ft)	Detention			
					Volume			
					(acre-ft)			
Detention	14.12	50.4%	0.674	0.244	1.162			
Facility								

5. Summary

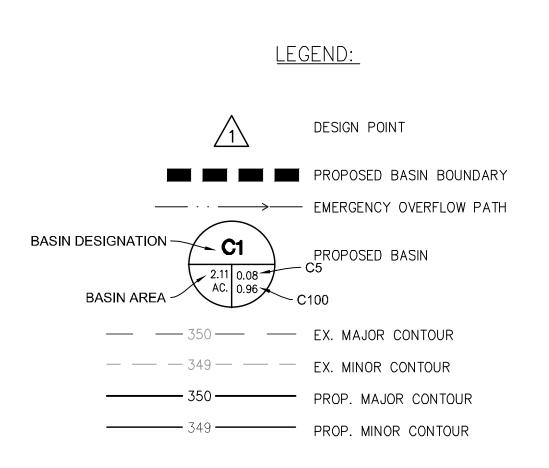
The proposed stormwater management design outlined in this report is in general conformance with the City of Lone Tree, Douglas County Storm Drainage Design and Technical Criteria Manual, and the Urban Drainage Manual. Stormwater runoff will continue to follow existing drainage patterns to Willow Creek and all undetained flows will bypass the proposed detention facility and continue to Willow Creek as they have historically done. The proposed detention facility will provide Water Quality, EURV, and 100-year detention for the majority of the proposed site. The anticipated imperviousness of the site is 50.4%. Outflows from the detention facility will be released at or below the allowable release rates and will not adversely affect Willow Creek or any downstream properties. There is also proposed drainage improvements along Willow Creek by others that are shown in Appendix G.

LIST OF APPENDICES

- Appendix A Drainage Basin Map
- Appendix B Hydrologic Computations
- Appendix C Soil Map
- Appendix D Detention Pond Sizing Spreadsheet
- Appendix E Referenced Information
- Appendix F Proposed Detention Facility Areas Map
- Appendix G Proposed Willow Creek Minimal Impact Drainage Plan

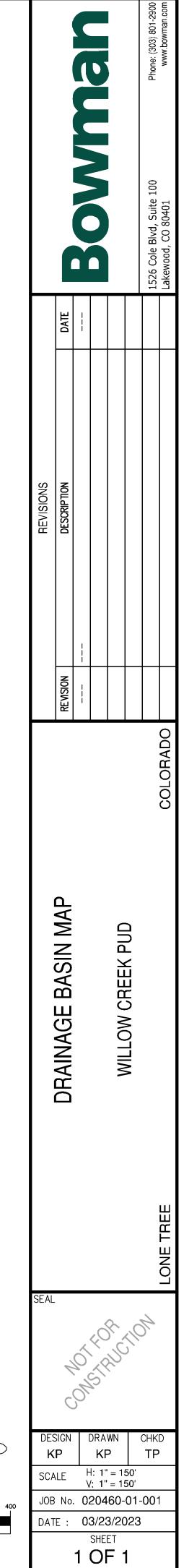
APPENDIX A – DRAINAGE BASIN MAP





ВА	SIN SUMM		
	Area	5-yr	100-yr
Basin	(acres)	(cfs)	(cfs)
A1	8.87	7.71	25.8
A2	4.49	7.60	20.9
UD1	0.76	0.12	2.3

	DESIGN POINT SUMMARY TABLE								
Design Point	Contributing Basins	Area (acres)	5-yr (cfs)	100-yr (cfs)					
1	A1	8.87	7.71	25.77					
2	A2	4.49	7.60	20.92					
3	A1, A2	13.36	12.95	40.21					
4	UD1	0.76	0.12	2.31					



GRAPHIC	SCALE

(IN FEET) 1 inch = 100 ft.

APPENDIX B – HYDROLOGIC COMPUTATIONS

Summary of Site Hydrology

Willow Creek PUD Lone Tree, CO

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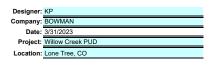
Tips for using spreadsheet

 if you need to override any values/equations in this spreadsheet - go to review tab, and choose "unprotect sheet" (this will make cells unlocked)
 These equations are based on Urban Drainage Volume 1 from January 2016 (please check urban drainage for most recent version) <u>http://udfcd.org/volume-one</u>

BASIN SUMMARY TABLE						
Basin	Area (acres)	5-yr (cfs)	100-yr (cfs)			
A1	8.87	7.71	25.8			
A2	4.49	7.60	20.9			
UD1	0.76	0.12	2.3			

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3	A1, A2	13.36	12.95	40.21					
4	UD1	0.76	0.12	2.31					

POST-DEVELOPMENT C VALUES



Global Parameters	
Land Use	% Imp.
Open Space/Landscaping	2
Hardscape	100
Roof	90

Summary						
14.12						
50.4%						



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¹ From Table 6-3 in UDFCD Volme 1

² From Table 6-4 in UDFCD Volme 1

Subcatchment Name	Area	NRCS Hydrologic Soil Group	Open Spac	e/Landscaping	Ha	ardscape	Roo	of	% Check	Percent			Rund	off Coefficie	ent, C ²		
Cuboutonnont Hamo	(ac)	intee injuiciegie een ereup	Area (ac)	%	Area (ac)	%	Area (ac)	%		Imperviousness	2-yr	5-yr	10-yr	25-yr	50-yr	100-yr	500-yr
A1	8.87	С	4.62	52.1%	2.70	30.4%	1.55	17.5%	100.00%	47.2%	0.36	0.42	0.48	0.58	0.63	0.68	0.74
A2	4.49	С	1.52	33.9%	2.12	47.2%	0.85	18.9%	100.00%	64.9%	0.51	0.56	0.61	0.68	0.71	0.75	0.79
UD1	0.76	C	0.76	100.0%	0.00	0.0%	0.00	0.0%	100.00%	2.0%	0.01	0.05	0.15	0.33	0.40	0.49	0.59

TIME OF CONCENTRATION

Designer:]			_					Bo	wm	an	
Date:	Company: BOWMAN Date: $3/31/2023$ $t_i = \frac{0.395(1.1 - C_5)\sqrt{10}}{S^{0.33}}$			$-C_5)\sqrt{L_i}$	Computed t_c	Computed $t_c = t_i + t_t$ $t_{minimum} = 10 (non-urban)$			C O N S U L T I N G Cells of this color are for required user-input						
-	Willow Cree Lone Tree,				t _t =	$=\frac{L_t}{60K\sqrt{S_t}}$	$=\frac{L_t}{60V_t}$	Regional t _c =	= (26 – 17i)	$+\frac{L_t}{60(14i+9)}$	$\sqrt{S_t}$ Sel	ected t _c = max{	t _{minimum} , mi	n(Computed	t _c , Regional t _c)}
	Subbasi	n Data		Overlan	d (Initial) Fl	low Time		Channe	ized (Travel)	Flow Time			Time of C	oncentration	
Sub-Basin	Area	% Impervious	C5	Overland Flow Length L _i (ft)	Overland Flow Slope S _i (ft/ft)	Overland Flow Time t _i (min)	Channelized Flow Length L _t (ft)	Channelized Flow Slope S _t (ft/ft)	NRCS Conveyanc e Factor K	Channelized Flow Velocity V _t (ft/sec)	Channelized Flow Time t _t (min)	Computed t _c (min)	Regional t _c (min)	Selected t _c (min)	
A1	8.87	47.2%	0.42	300.00	0.030	14.80	1000.00	0.020	17	2.40	6.93	21.74	25.52	21.74	
A2	4.49	64.9%	0.56	100.00	0.025	7.15	450.00	0.020	20	2.83	2.65	9.80	17.89	9.80	
UD1	0.76	2.0%	0.05	150.00	0.250	8.02	0.00	0.010	20	2.00	0.00	8.02	25.66	10.00	

STORM DRAINAGE SYSTEM DESIGN - 5-YEAR DESIGN STORM



Designer:	KP
Company:	BOWMAN
Date:	3/31/2023
Project:	Willow Creek PUD
Location:	Lone Tree, CO

CONSULTING
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	STREET/	DIRECT RUNOFF						TOTAL RUNOFF				STR			PIPE						TRAVEL	TIME		
DESGIN POINT	CONTRIBUTING	Area	Area	Coeff C		C*A	I	Q	Tc (min)	Sum Area	Sum C*A	l in/hr	Q cfs	Slope %	Street Q cfs	Design Q cfs	Slope %	PIPE SIZE	L	VEL ft/sec	Tt min	Q add'l		Remarks
		Design		-	(min)	(ac)		(cfs)	(min)	(ca)	(ac)	Invnr	cis	70	CIS		70	SIZE	π	n/sec	min			
		A1	8.87	0.42	21.7	3.72	2.07	7.7								7.7								
1	A1								21.7	8.9	3.7	2.07	7.71			7.7								
		A2	4.49	0.56	9.8	2.53	3.00	7.6								7.6								
2	A2								9.8	4.5	2.5	3.00	7.60			7.6								
		UD1	0.76	0.05	10.0	0.04	2.98	0.1								0.1								
3	A1, A2								21.7	13.4	6.3	2.07	12.95			13.0								
4	UD1								10.0	0.8	0.0	2.98	0.12			0.1								

STORM DRAINAGE SYSTEM DESIGN - 100-YEAR DESIGN STORM



Cells of this color are for required user-input Cells of this color are for optional user-input

Designer:	KP
Company:	BOWMAN
Date:	3/31/2023
Project:	Willow Creek PUD
Location:	Lone Tree, CO

STREET/		DIRECT RUNOFF						TO	TAL RUN	OFF			EET		PIPE						TRAVEL	TIME		
DESGIN POINT	CONTRIBUTING	Area	Area	Coeff	Tc	C*A	I	Q	Tc	Sum Area	Sum C*A	1				Design Q		PIPE	L	VEL	Tt	Q add'l		Remarks
		Design		С	(min)	(ac)		(cfs)	(min)	(ca)	(ac)	in/hr	cfs	%	cfs	cfs	%	SIZE	ft	ft/sec	min			
		A1	8.87	0.68	21.7	6.01	4.29	25.8								25.8								
1	A1								21.7	8.9	6.0	4.29	25.77			25.8								
		A2	4.49	0.75	9.8	3.37	6.22	20.9								20.9								
2	A2								9.8	4.5	3.4	6.22	20.92			20.9								
		UD1	0.76	0.49	10.0	0.37	6.17	2.3								2.3								
3	A1, A2								21.7	13.4	9.4	4.29	40.21			40.2								
4	UD1								10.0	0.8	0.4	6.17	2.31			2.3								
													-											
							-																	
				l																				
1																								

Rainfall Data Willow Creek PUD Lone Tree, CO

Recurrence Interval (yrs)	1-hr Rainfall Depth (in)					
2	0.84					
5	1.10					
10	1.33					
25	1.68					
50	1.97					
100	2.28					
500	3.08					

APPENDIX C – SOIL MAP



National Cooperative Soil Survey

Conservation Service

Page 1 of 3

Μ	AP LEGEND	MAP INFORMATION				
Area of Interest (AOI) Area of Interest (Soils	AOI) Stony Spot (1) Very Stony Spot	The soil surveys that comprise your AOI were mapped at 1:20,000. Warning: Soil Map may not be valid at this scale.				
Soil Map Unit Po Soil Map Unit Lir Soil Map Unit Po Soil Map Unit Po Special Point Features	ygons ver Wet Spot es △ Other nts Special Line Features	Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.				
BlowoutBorrow PitClay Spot	Water Features Streams and Canals Transportation HHH Rails	Please rely on the bar scale on each map sheet for map measurements. Source of Map: Natural Resources Conservation Service				
Closed Depressi		Web Soil Survey URL: Coordinate System: Web Mercator (EPSG:3857) Maps from the Web Soil Survey are based on the Web Mercator				
Gravelly Spot	Major Roads Local Roads Background	projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required. This product is generated from the USDA-NRCS certified data as				
Marsh or swamp Mine or Quarry Miscellaneous W	Aerial Photography	of the version date(s) listed below. Soil Survey Area: Castle Rock Area, Colorado Survey Area Data: Version 15, Sep 1, 2022				
 Perennial Water Rock Outcrop Saline Spot 		Soil map units are labeled (as space allows) for map scales 1:50,000 or larger. Date(s) aerial images were photographed: Jun 9, 2021—Jun 1: 2021				
Sandy Spot Severely Eroded	Spot	The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.				
Slide or Slip Sodic Spot						



Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
FoD	Fondis clay loam, 3 to 9 percent slopes	84.2	21.3%
Ма	Manzanola clay loam	11.8	3.0%
NsE	Newlin-Satanta complex, 5 to 20 percent slopes	27.7	7.0%
RmE	Renohill-Buick complex, 5 to 25 percent slopes	242.6	61.3%
Sn	Satanta loam	29.5	7.4%
Totals for Area of Interest	·	395.7	100.0%

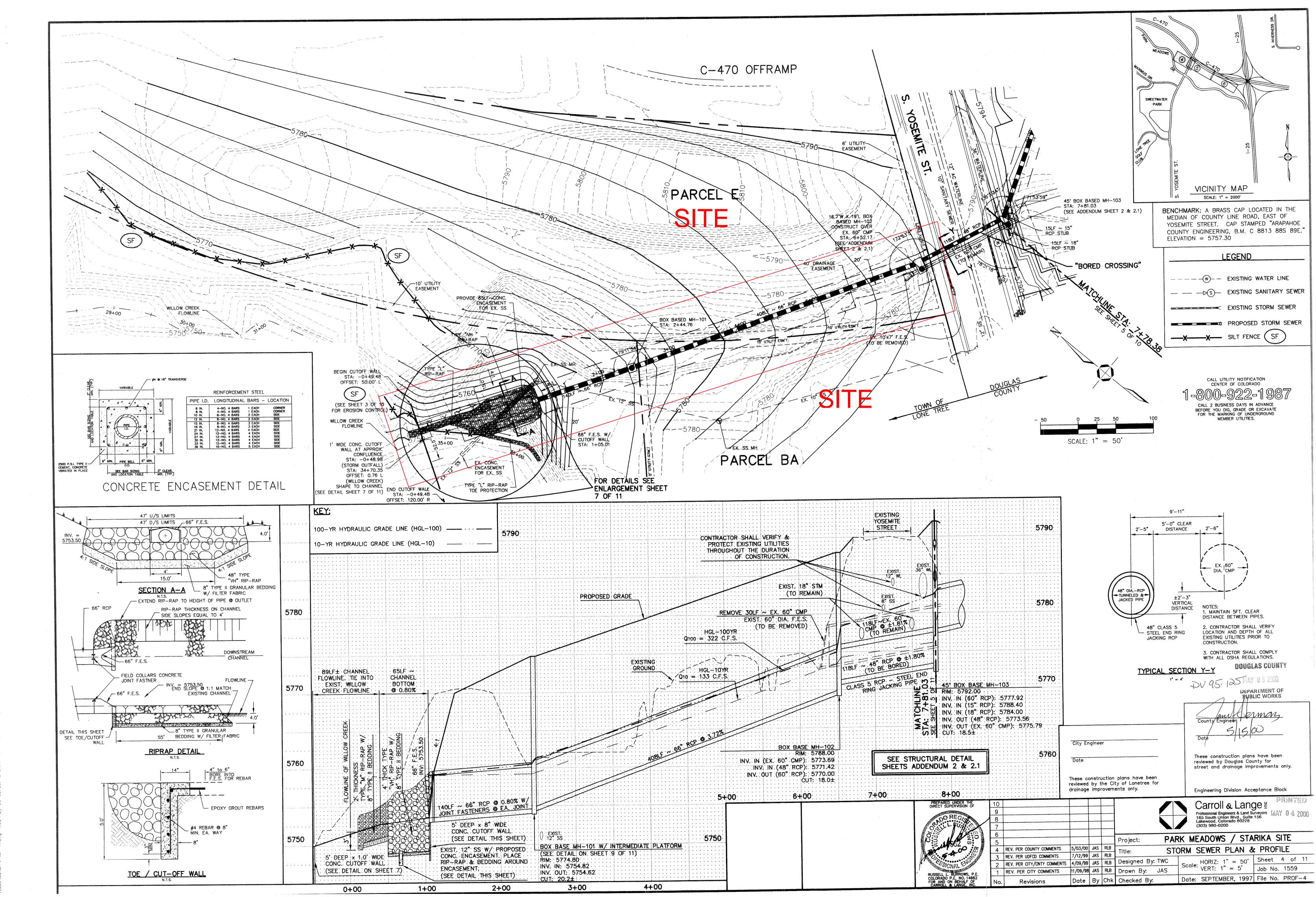


APPENDIX D – DETENTION POND SIZING SPREADSHEET

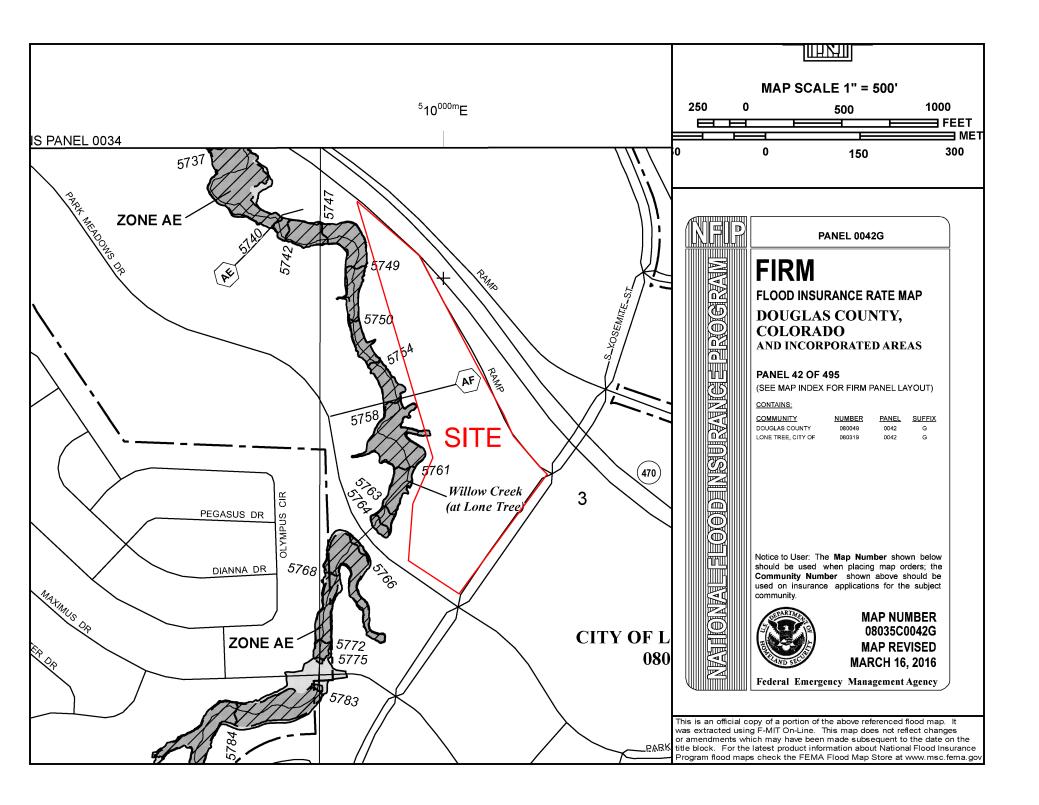
DETENTION BASIN STAGE-STORAGE TABLE BUILDER

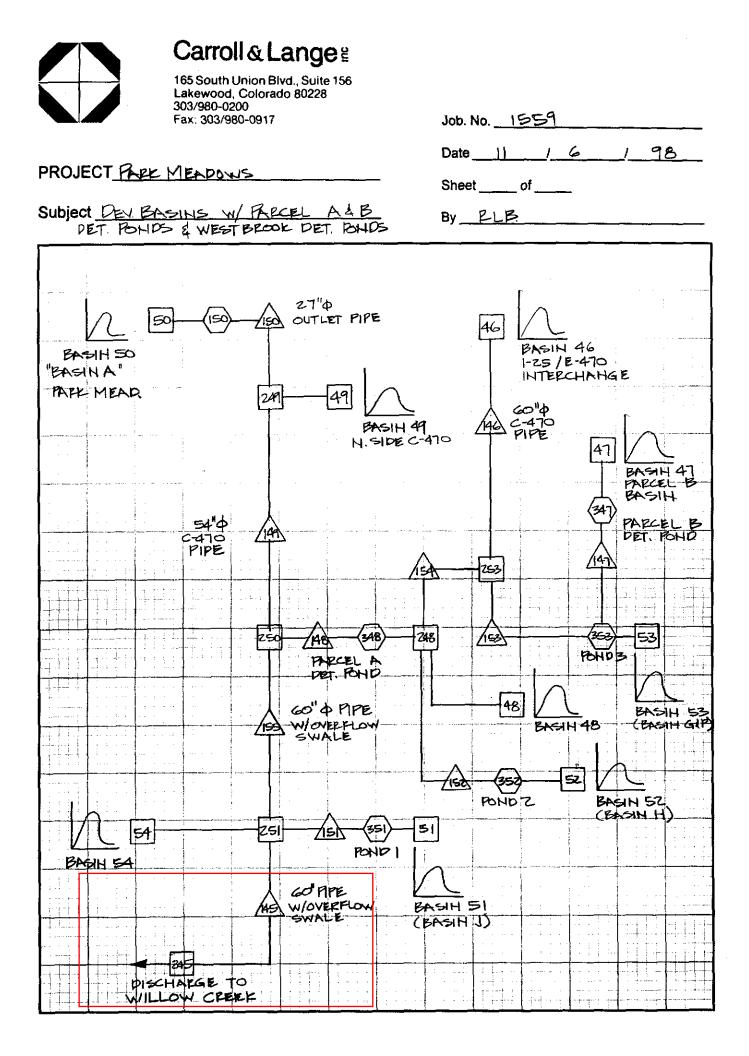
Project:				MHFD-D	etention, Version	4.04 (Febr	ruary 2021))						
Basin ID:														
ZONE 3	the s													
TT work		100-YEA ORIFICE	R		Depth Increment =		<u>م</u>							
PERMANENT ORIFICI POOL Example Zono							Optional		Mr. HL	Area	Optional Override		Volume	
POOL Example Zone	Configura	tion (Reten	tion Pond)		Stage - Storage Description	Stage (ft)	Override Stage (ft)	Length (ft)	Width (ft)	(ft ²)	Area (ft ²)	Area (acre)	(ft ³)	Volume (ac-ft)
Watershed Information		1			Top of Micropool									
Selected BMP Type = Watershed Area =	EDB 14.12	acres												
Watershed Length =	800	ft												
Watershed Length to Centroid =	500	ft												
Watershed Slope = Watershed Imperviousness =	0.025 50.40%	ft/ft percent												
Percentage Hydrologic Soil Group A =	0.0%	percent												
Percentage Hydrologic Soil Group B =	0.0%	percent												
Percentage Hydrologic Soil Groups C/D = Target WQCV Drain Time =	100.0% 40.0	percent hours												
Location for 1-hr Rainfall Depths = L	Lone Tree - M	unicipal Cour	t											
After providing required inputs above incli depths, click 'Run CUHP' to generate runo	luding 1-hour	rainfall s using												
the embedded Colorado Urban Hydrog			Optional Use	r Overrides										
Water Quality Capture Volume (WQCV) =	0.244	acre-feet		acre-feet										
Excess Urban Runoff Volume (EURV) = 2-yr Runoff Volume (P1 = 0.84 in.) =	0.674	acre-feet acre-feet	0.84	acre-feet inches										
5-yr Runoff Volume (P1 = 1.1 in.) =	0.647	acre-feet	1.10	inches										
10-yr Runoff Volume (P1 = 1.33 in.) =	0.876	acre-feet	1.33	inches										
25-yr Runoff Volume (P1 = 1.68 in.) = 50-yr Runoff Volume (P1 = 1.97 in.) =	1.322	acre-feet acre-feet	1.68	inches inches										
100-yr Runoff Volume (P1 = 2.28 in.) =	2.055	acre-feet	2.28	inches										
500-yr Runoff Volume (P1 = 3.08 in.) =	3.009	acre-feet	3.08	inches		-			-					
Approximate 2-yr Detention Volume = Approximate 5-yr Detention Volume =	0.419	acre-feet acre-feet												
Approximate 10-yr Detention Volume =	0.767	acre-feet												
Approximate 25-yr Detention Volume =	0.916	acre-feet												
Approximate 50-yr Detention Volume = Approximate 100-yr Detention Volume =	0.989	acre-feet acre-feet												
Define Zones and Basin Geometry]												
Select Zone 1 Storage Volume (Required) = Select Zone 2 Storage Volume (Optional) =		acre-feet acre-feet												
Select Zone 3 Storage Volume (Optional) =		acre-feet												
Total Detention Basin Volume = Initial Surcharge Volume (ISV) =		acre-feet ft ³												
Initial Sucharge Volume (ISV) =		ft												
Total Available Detention Depth (H _{total}) =		ft												
Depth of Trickle Channel $(H_{TC}) =$ Slope of Trickle Channel $(S_{TC}) =$		ft ft/ft												
Slopes of Main Basin Sides (S _{main}) =		H:V												
Basin Length-to-Width Ratio $(R_{L/W}) =$														
Initial Surcharge Area (A _{ISV}) =		ft 2												
Surcharge Volume Length $(L_{ISV}) =$		ft												
Surcharge Volume Width (W _{ISV}) = Depth of Basin Floor (H _{FLOOR}) =		ft ft											 	
Length of Basin Floor $(L_{FLOOR}) =$		ft												
Width of Basin Floor (W_{FLOOR}) =		ft												
Area of Basin Floor $(A_{FLOOR}) =$ Volume of Basin Floor $(V_{FLOOR}) =$		ft ² ft ³												
Depth of Main Basin (H _{MAIN}) =		ft												
Length of Main Basin (L _{MAIN}) =		ft												
Width of Main Basin (W _{MAIN}) = Area of Main Basin (A _{MAIN}) =		ft ft ²												
Volume of Main Basin (V _{MAIN}) =		ft ³												
Calculated Total Basin Volume (V_{total}) =	-	acre-feet				-			-					
					LI		1			1		l .		

APPENDIX E – REFERENCED INFORMATION



9\CONST\PROF-4.dwg Wed May 03 15:21:49 2000 CARROLL & LANGE, INC.

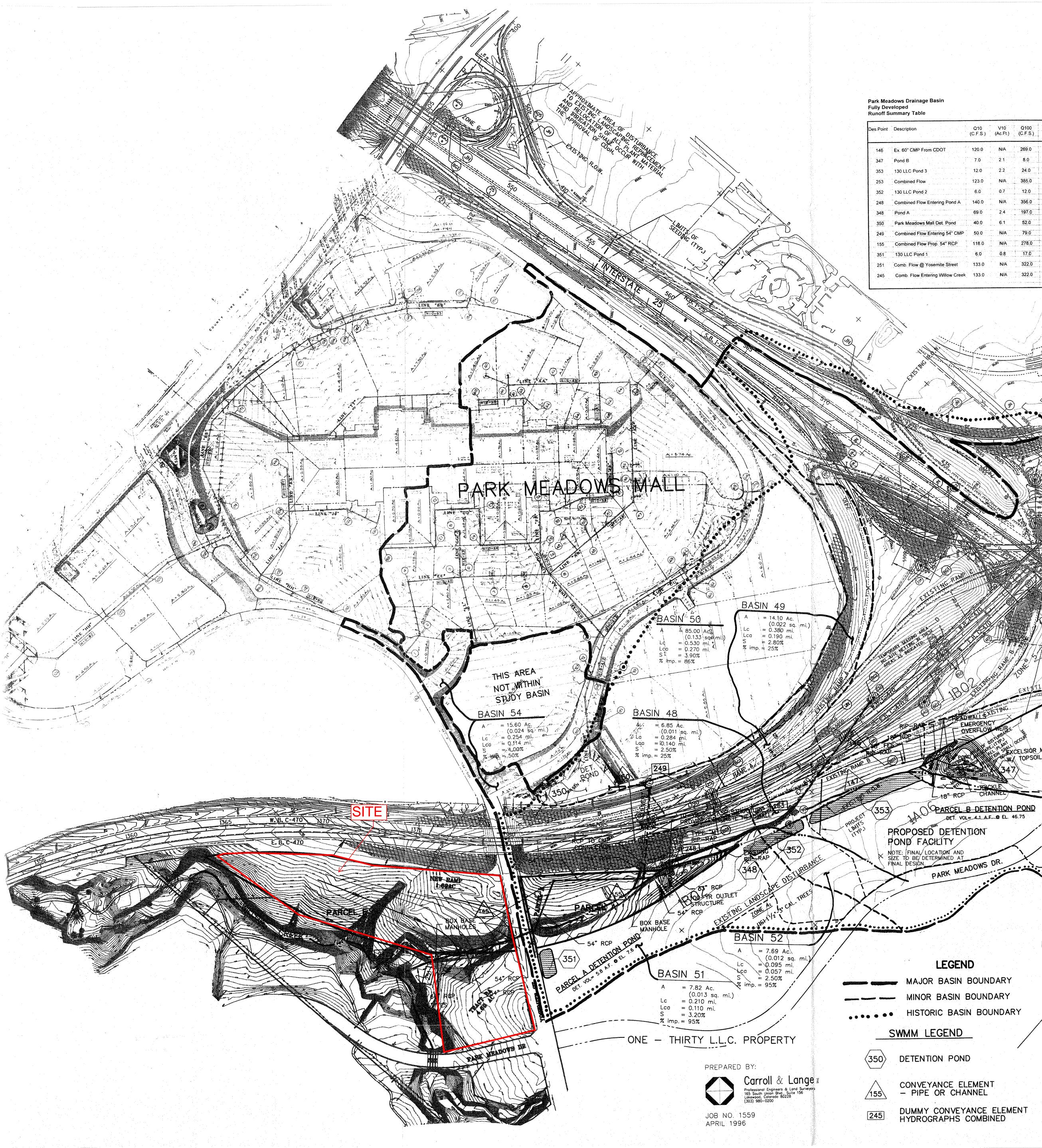




DOUGLAS COUNTY - WILLOW CREEK 100-YR DEV. COND., WITH PK MEAD/E470 BASINS/STRAM CARROLL & LANGE, INC. HYD.FILE: 1559DEHP.HY4

PEAK (CFS) TIME (HR/MIN) STAGE (FT) STORAGE (AC-FT) CONVEYANCE ELEMENT 35. 35. 35. 40. 108. (DIRECT FLOW) 0 47 8. 347 53 147 353 46 52 153 153 153 152 50 152 4.1 10110010 . 1 (DIRECT FLOW) 9Ì. 8. .4 .1 24. 3.2 (DIRECT FLOW) (DIRECT FLOW) (DIRECT FLOW) 5.0 35. 30. 15. 289. 64. 24. 269. 12. 385. .3 **4**0. 55. 45. 1 00000010011001000000 .2 (DIRECT FLOW) (DIRECT FLOW) (DIRECT FLOW) (DIRECT FLOW) (DIRECT FLOW) 2.7 422. 12. 19. 35. 55. 35. 35. 48 154 328. 52. (DIRECT FLOW) (DIRECT FLOW) 1.3 20 35. 350 248 356. 40. 20. 49 150 348 249 51 148 149 36. <u>52</u>. 197. 79. 71. Ô. .i 5.8 (DIRECT FLOW) (DIRECT FLOW) (DIRECT FLOW) 5Ò. 30. 197. 0. 45. 50. 80. 1.4 351 17. .1 1.3 250 54 151 155 251 145 245 275. 62. 17. (DIRECT FLOW) (DIRECT FLOW) 55. 35. 50. (DIRECT FLOW) 3.5 (DIRECT FLOW) 278. 322 322. 322. 322. 55. 55 0 55. 55. Ō 0

*** PEAK FLOWS, STAGES AND STORAGES OF GUTTERS AND DETENSION DAMS ***



Q10 (C.F.S.)	V10 (Ac.Ft.)	Q100 (C.F.S.)	V100 (Ac.Ft.)
120.0	N/A	269.0	N/A
7.0	2.1	8.0	4.1
12.0	2.2	24.0	3.2
123.0	N/A	385.0	N/A
6.0	0.7	12.0	1.2
140.0	N/A	356.0	N/A
69.0	2.4	197.0	5.8
40.0	6.1	52.0	12.7
50.0	N/A	79.0	N/A
118.0	N/A	278.0	N/A
6.0	0.8	17.0	1.3
133.0	N/A	322.0	N/A
133.0	N/A	322.0	N/A

HISTORIC BASIN AREA = ± 200 AC. DEV./EXIST. BASIN AREA = ± 270 AC.

PARCEL B BASIN 47

BASIN-53

= 19.01 Ac. (0.030 sq. mi.) = 0.380 mi. $_{co}$ = 0.190 mi. S = 3.60% % imp. = 95%

SEXCELSION MATTING

BASIN 46

= 24,00 Aq= (0.038 sq. mi.)

(0.000 sy)= 0.416 mi. 0.200 mi.S = 4.50% % imp: = 80%

= 88.60 Ac

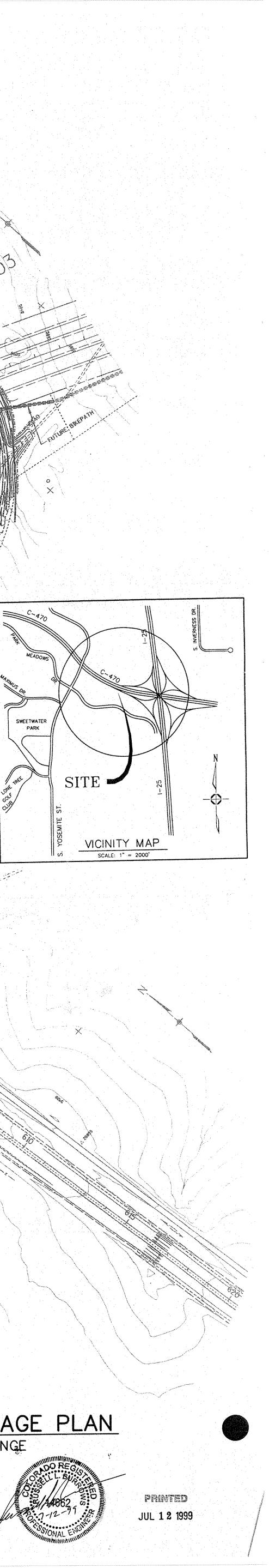
NOTE: EXISTING MAPPING PREPARED FROM A COMPOSITE OF: PARK MEADOWS MALL DRAINAGE PLAN CDOT C-470 / 1-25 INTERCHANGE GRADING PLANS WESTERN STATES SURVEYING TOPOGRAPHY OF STARIKA PROPERTY & WILLOW CREEK

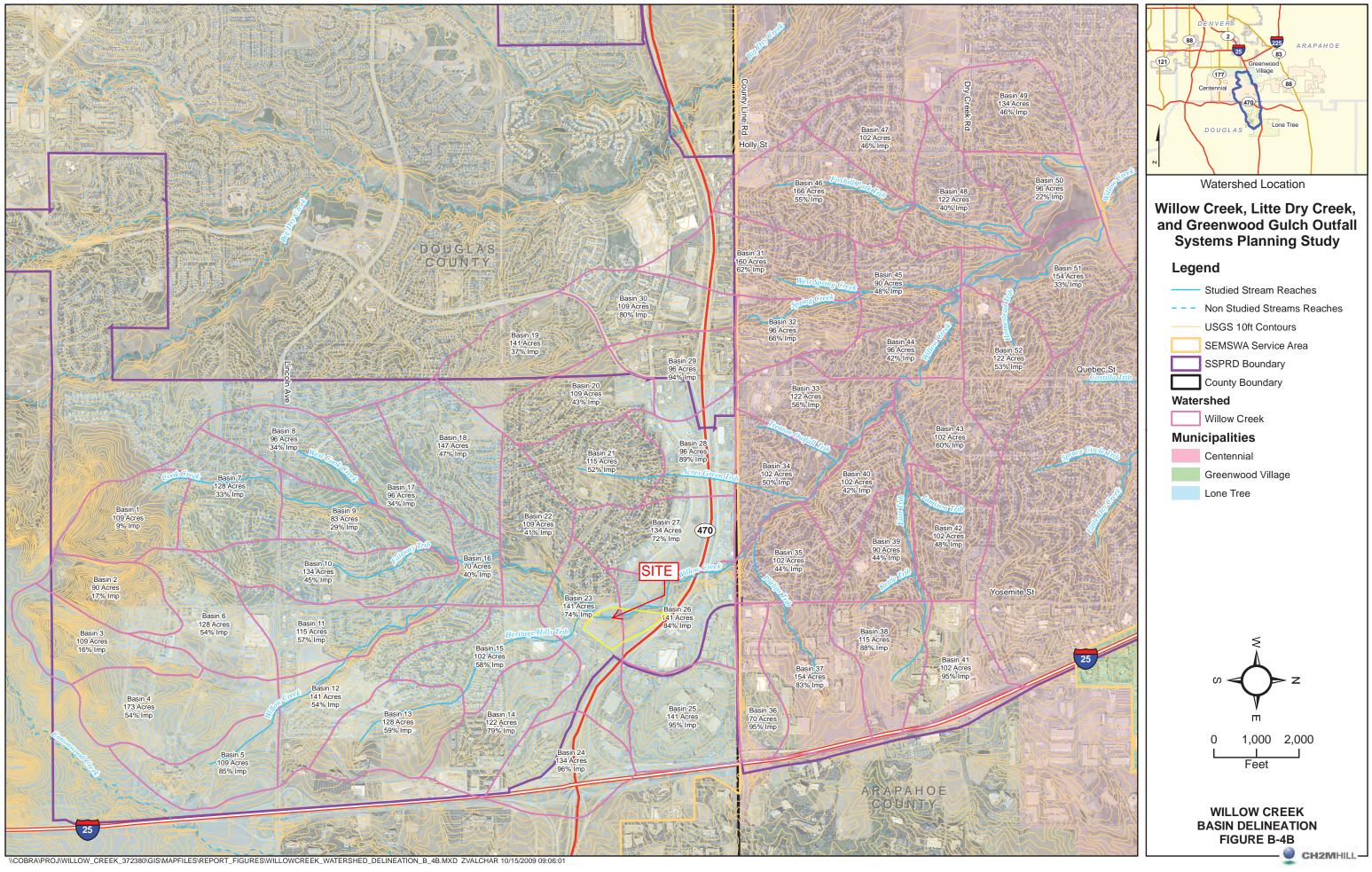
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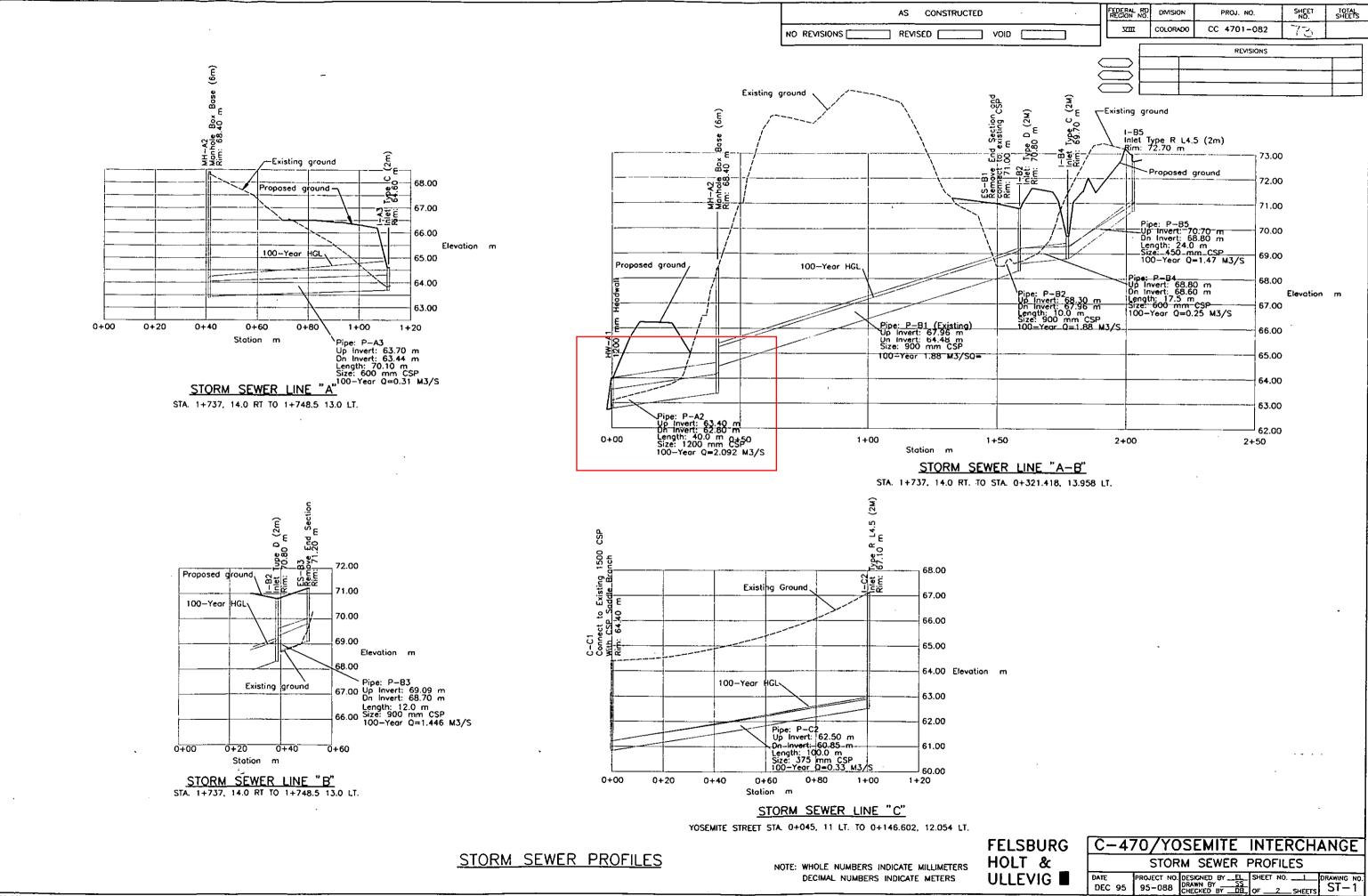
FUTURE DEVELOPED CONDITION DRAINAGE PLAN @ PARK MEADOWS AND C-470/1-25 INTERCHANGE

REV. 10/27/97 - UPDATED DRAINAGE PLAN INCLUDED 130LLC BASINS.

REV. 11/09/98 - UPDATED DRAINAGE PLAN FINAL DESIGN INCLUDING 130LLC BASINS.





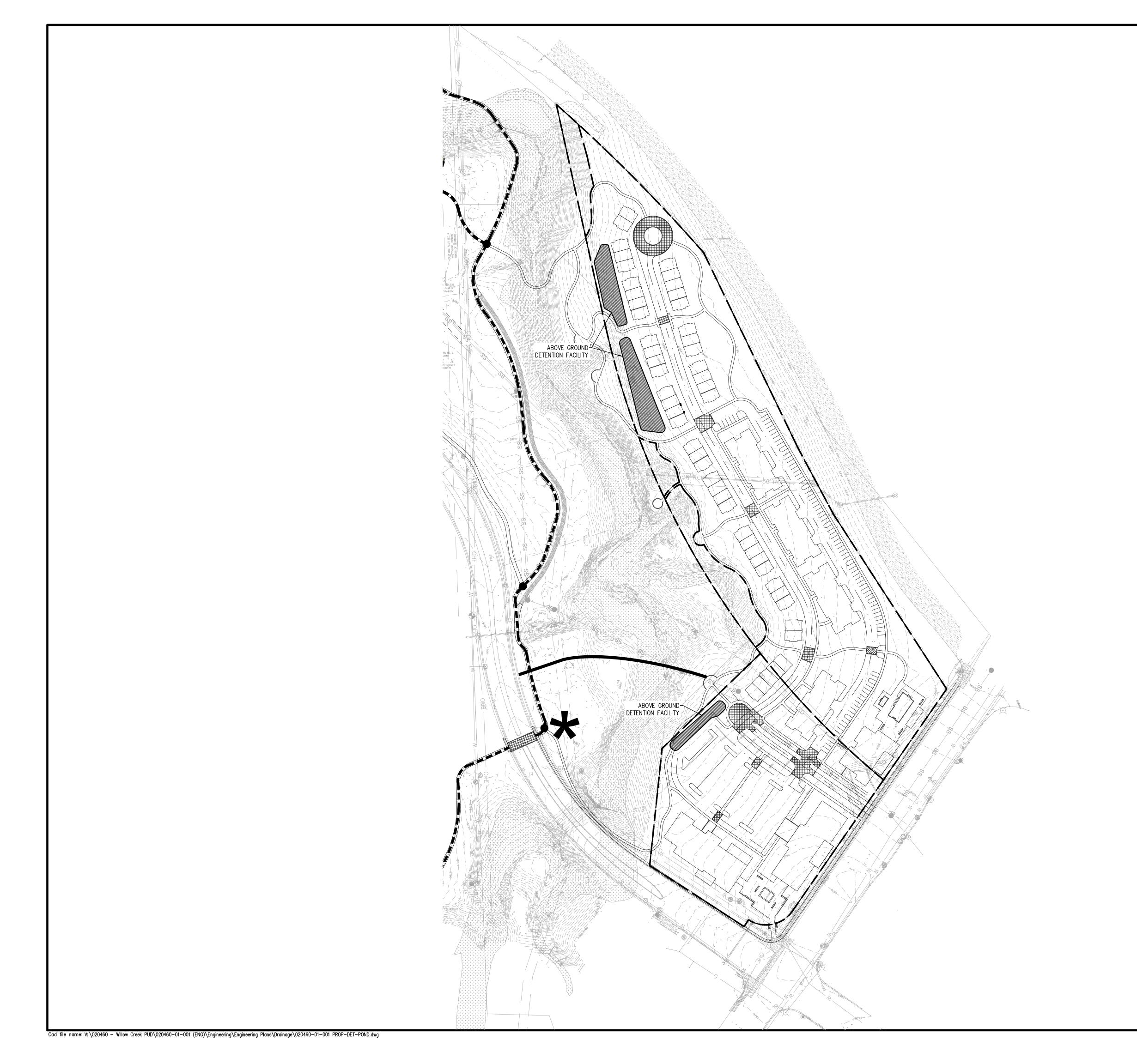


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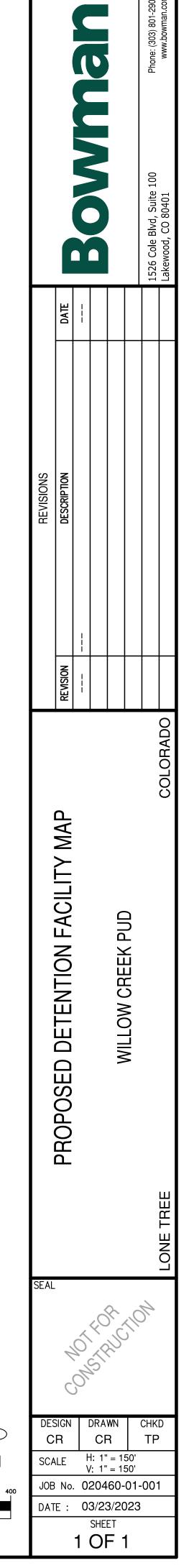
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COMPUTER GENERATED DRAWING DO 1.07 ALTER MANUALLY

APPENDIX F- PROPOSED DETENTION FACILITY AREAS MAP



LEGEND: DESIGN POINT PROPOSED BASIN BOUNDARY PROPOSED BASIN BOUNDARY EMERGENCY OVERFLOW PATH 350 EX. MAJOR CONTOUR 350 PROP. MAJOR CONTOUR 349 PROP. MAJOR CONTOUR PROP. MAJOR CONTOUR PROP. MAJOR CONTOUR PROP. ABOVE GROUND DETENTION FACILITY



GRAPHIC	SCAL

(IN EFET)

(IN FEET) 1 inch = 100 ft.

APPENDIX G- WILLOW CREEK MINIMAL IMPACT DRAINAGE PLAN

